

## SITE VIEW



**2328 Florence Avenue**  
**Cincinnati, OH 45206**  
**(513) 241-6775 | FAX (513) 241-5060**  
**atlanticsigncompany.com**

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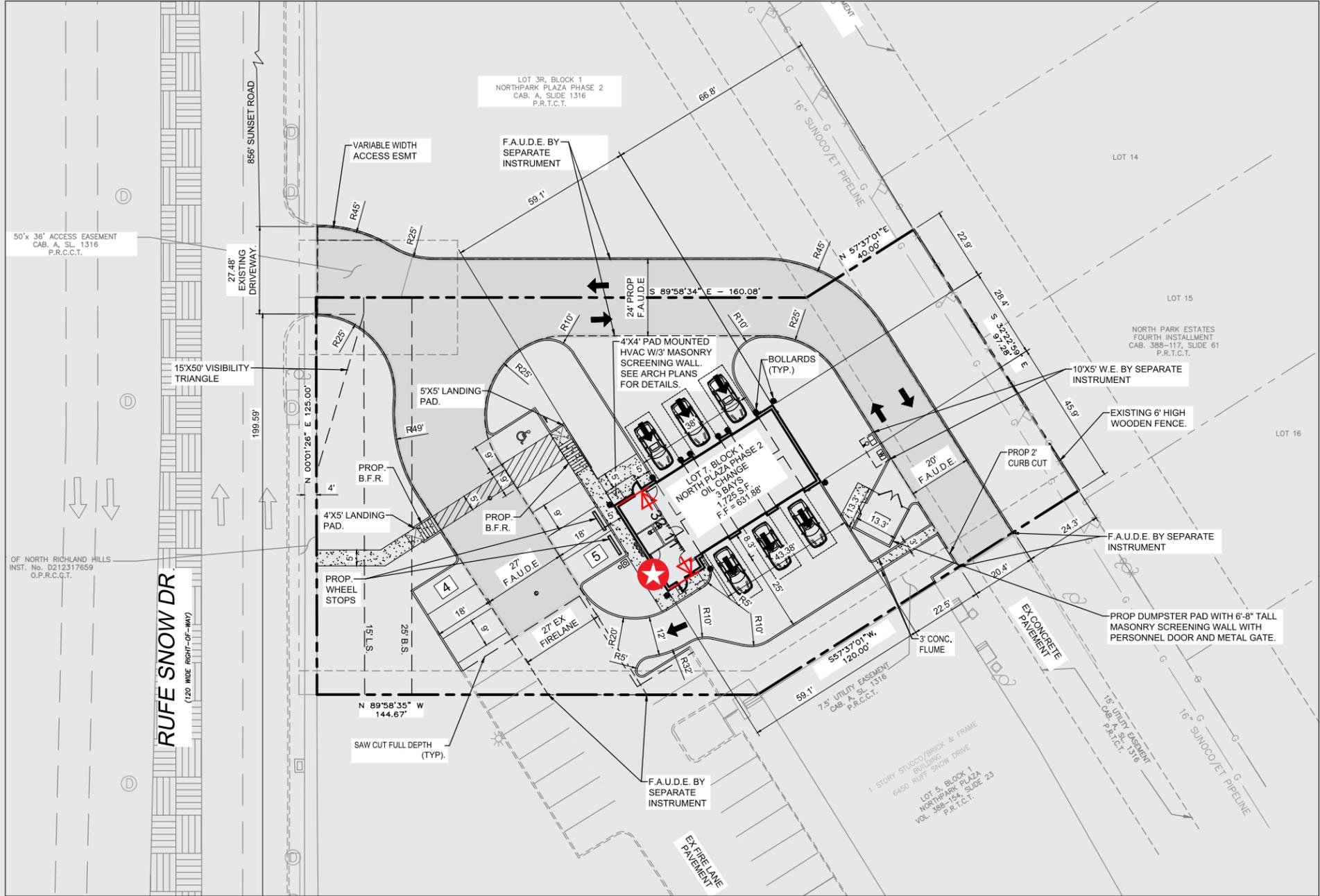


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SIGNATURE \_\_\_\_\_ DATE \_\_\_\_\_

CUSTOMER	STRICKLAND BROTHERS - NORTH RICHLAND HILLS, TX
LOCATION	6450 RUFÉ SNOW, NORTH RICHLAND HILLS, TX, 76182
SALES REP	DAVID ESTEP
PROJECT MGR.	JESSICA REYNOLDS
DRAWN BY	RYAN PARKER
DATE	3-1-2024
FILE NAME:	107031 STRICKLAND BROTHERS_N RICHLAND HILLS_TX_WEST PERMIT

# NEW LOGO SIGN TO BE ADDED TO TOWER



## SITE VIEW



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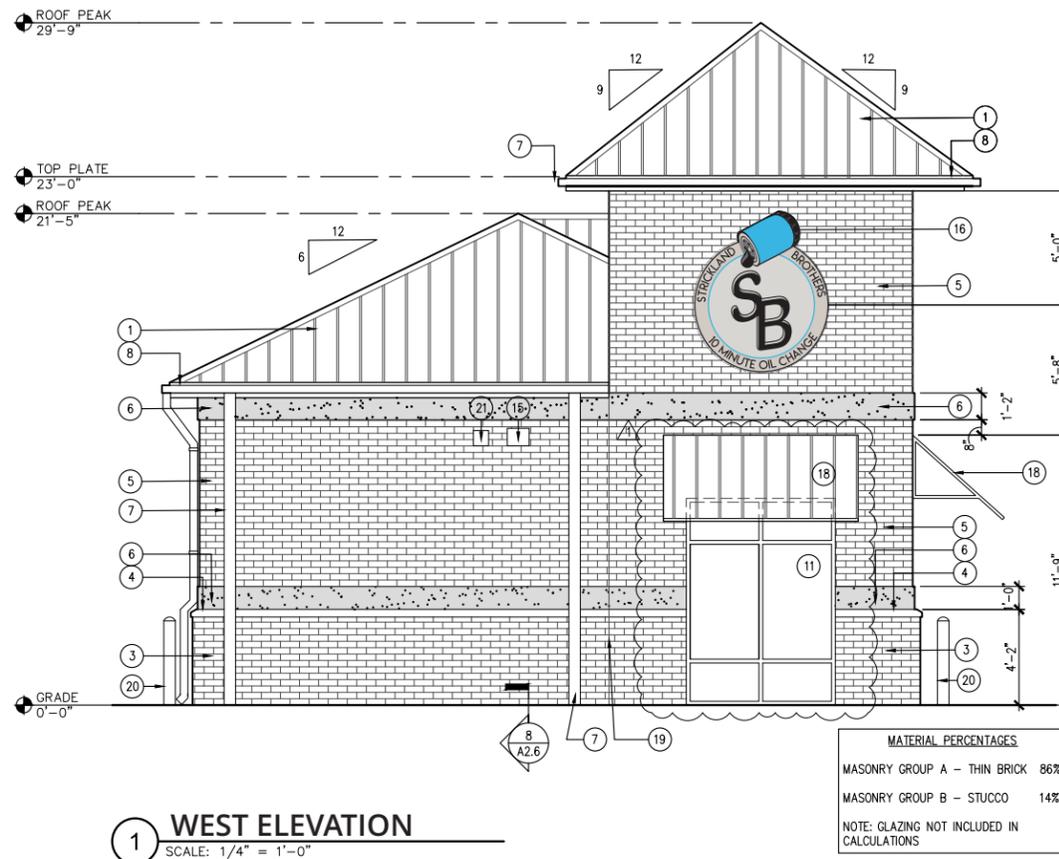
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**1 WEST ELEVATION**  
SCALE: 1/4" = 1'-0"

# PROPOSED



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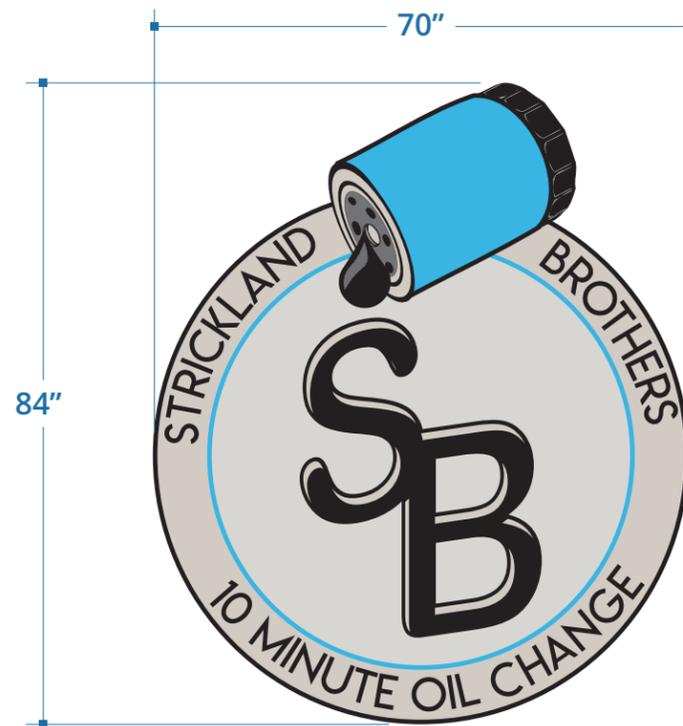
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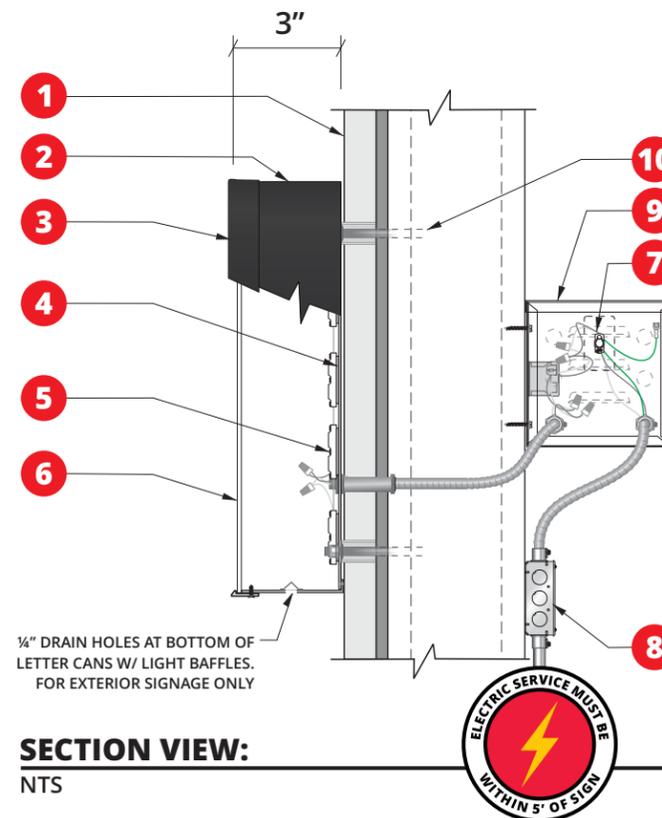


**FRONT:** 41 SQUARE FEET • **QUANTITY 1**  
1/2" SCALE

**SPECIFICATIONS: ILLUMINATED WALL LOGO**

- 1 EXISTING FACADE
- 2 .040" ALUMINUM LETTER RETURNS PAINTED BLACK
- 3 1" TRIMCAP BONDED TO FACE, #8 PAN HEAD SCREWS TO RETURNS - PAINTED BLACK  
NOTE: 2" TRIMCAPS FOR LETTERS 54" AND LARGER
- 4 .125" ACM BACKS (PRE-FINISHED WHITE) FASTENED TO RETURNS.  
SEAL W/ VOC COMPLIANT 360 WHITE LATEX CAULK TO PREVENT MOISTURE PENETRATION.
- 5 WHITE LED MODULES
- 6 3/16" WHITE ACRYLIC FACES WITH TRANSLUCENT VINYL PRINT
- 7 DISCONNECT SWITCH UL OUTDOOR RATED TOGGLE TYPE W/ NEOPRENE BOOT PER NEC 600-6
- 8 PRIMARY ELECTRICAL FEED IN 1/2" FLEX UL CONDUIT / CUSTOMER SUPPLIED UL JUNCTION BOX
- 9 POWER SUPPLIES WITHIN UL ENCLOSURE (REMOVABLE LID), 1/4" X 1" MIN SCREWS

	TYPE	DIA.	SHEAR LBS.	EMBED	SPACING	FOR USE WITH
10	THREADED ROD WITH SILICONE ADHESIVE	1/4"	900	4"-6"	16" *MINIMUM OF 2 FASTENERS PER UNIT	CONCRETE, BRICK, CONCRETE BLOCK, OR EIFS



**COLORS USED**

Digitally Printed Vinyl	Paint
PMS 298c (Logo)	PMS COOL GRAY 10c (Logo)
PMS 427c (Logo)	BLACK 100% (Logo)
PMS 420c (Logo)	PMS 420c (Face & Returns)

**GENERAL NOTES:**

- THIS SIGN IS TO BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS OF ARTICLE 600 OF THE NATIONAL ELECTRICAL CODE.
- GROUNDED AND BONDED PER NEC 600.7/NEC 250
  - EXISTING BRANCH CIRCUIT IN COMPLIANCE WITH NEC 600.5, NOT TO EXCEED 20 AMPS
  - SIGN IS TO BE UL LISTED PER NEC 600.3
  - UL DISCONNECT SWITCH PER NEC 600.6- REQUIRED PER SIGN COMPONENT BEFORE LEAVING MANUFACTURER. FOR MULTIPLE SIGNS, A DISCONNECT IS PERMITTED BUT NOT REQUIRED FOR EACH SECTION
  - THE LOCATION OF THE DISCONNECT SWITCH AFTER INSTALLATION SHALL COMPLY WITH ARTICLE 600.6 (A) (1) PER NEC



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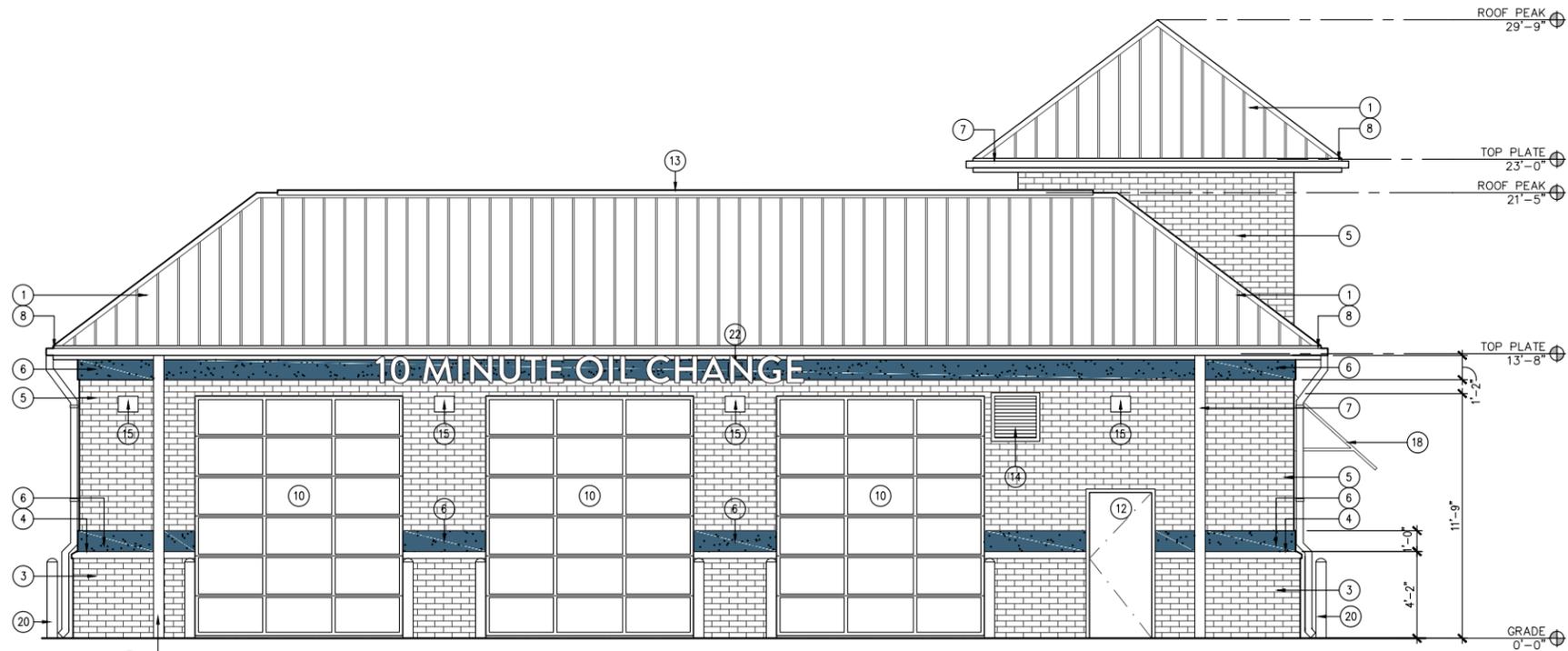
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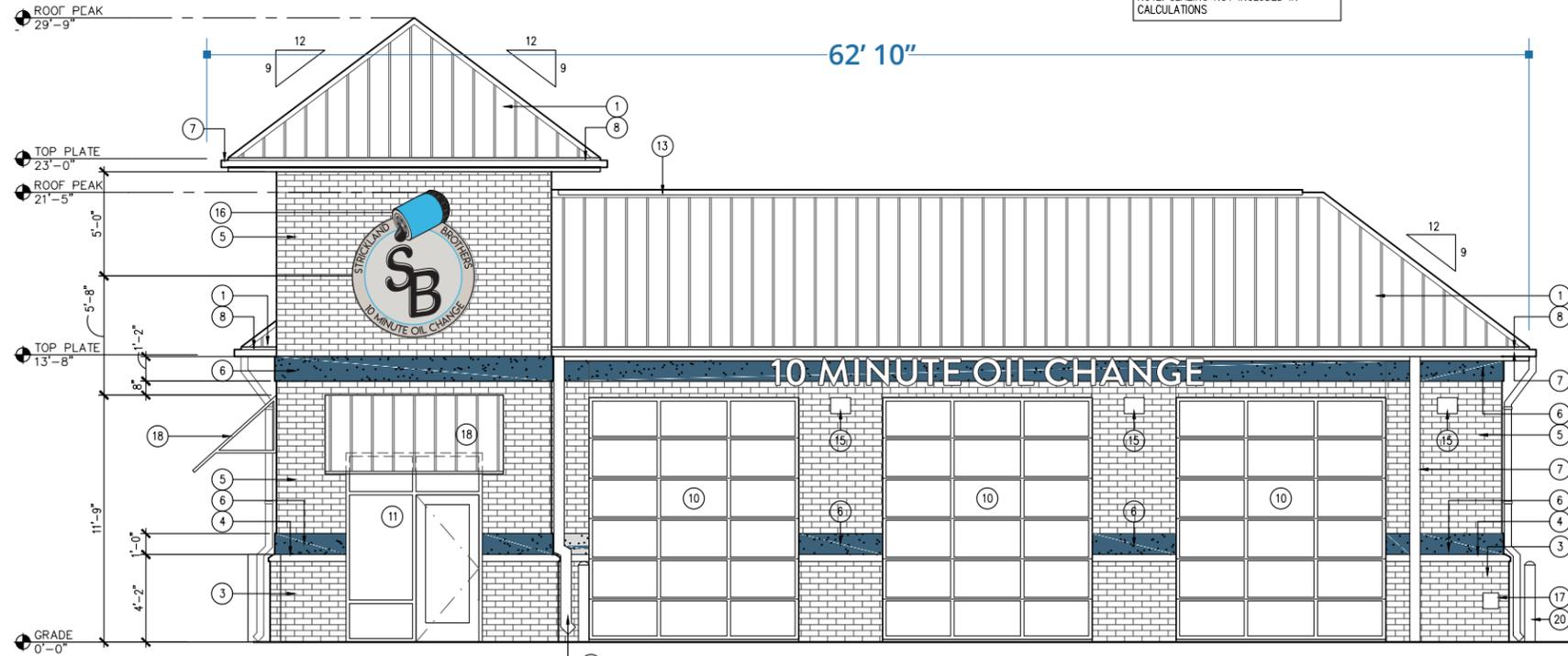




**2 NORTH ELEVATION**  
SCALE: 1/4" = 1'-0"

MATERIAL PERCENTAGES	
MASONRY GROUP A - THIN BRICK	80%
MASONRY GROUP B - STUCCO	20%
NOTE: GLAZING NOT INCLUDED IN CALCULATIONS	

- PMS 298 C  
EXACT VINYL 3630-057 OLYMPIC BLUE
- PMS 427 C  
EXACT VINYL COLOR TBD
- PMS 420 C  
EXACT VINYL COLOR TBD
- PMS COOL GRAY 10 C  
EXACT VINYL COLOR TBD
- SW 9149 INKY BLUE  
PMS 5405 C  
SHADOW BLUE 7725-177



**2 SOUTH ELEVATION**  
SCALE: 1/4" = 1'-0"

MATERIAL PERCENTAGES	
MASONRY GROUP A - THIN BRICK	82%
MASONRY GROUP B - STUCCO	18%
NOTE: GLAZING NOT INCLUDED IN CALCULATIONS	

# PROPOSED



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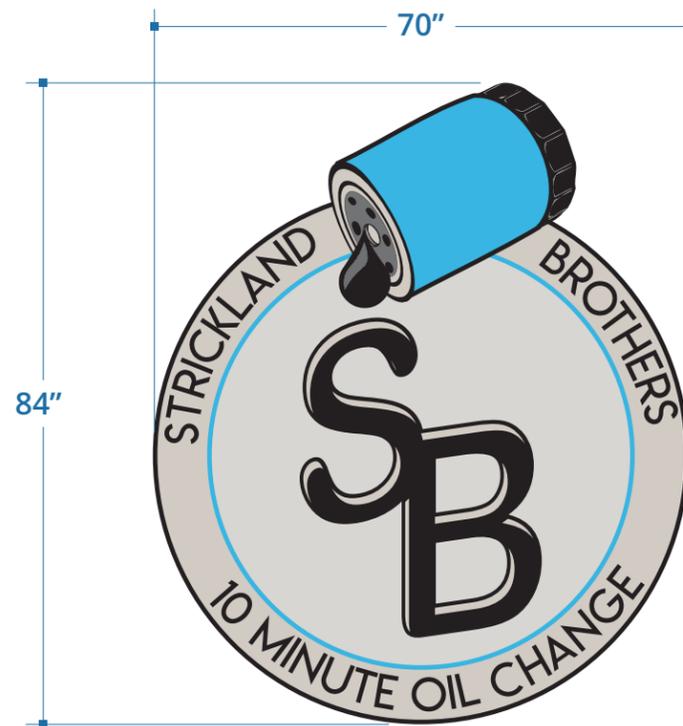


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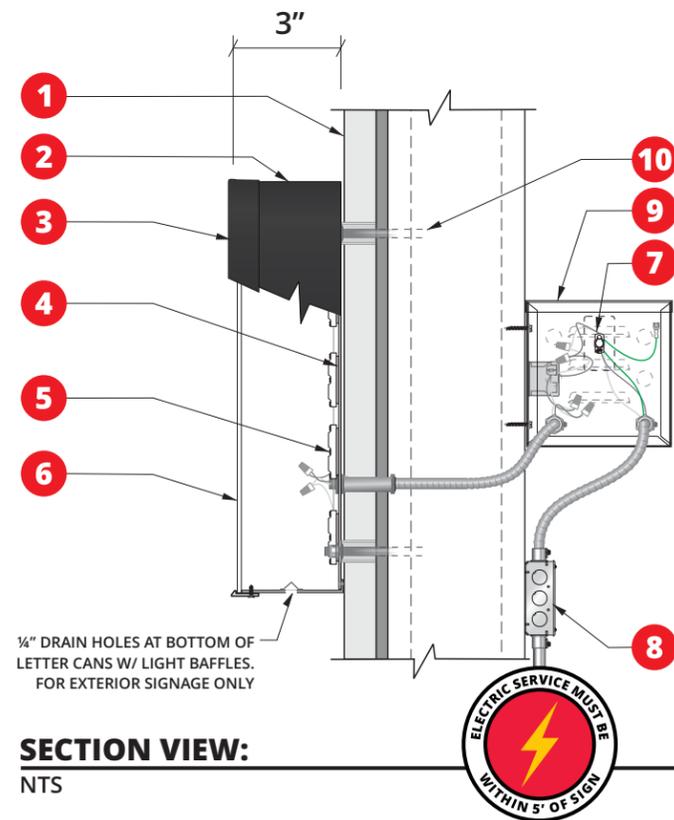


**FRONT:** 41 SQUARE FEET • **QUANTITY 1**  
**1/2" SCALE**

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- 9 POWER SUPPLIES WITHIN UL ENCLOSURE (REMOVABLE LID), 1/4" X 1" MIN SCREWS

	TYPE	DIA.	SHEAR LBS.	EMBED	SPACING	FOR USE WITH
10	 THREADED ROD WITH SILICONE ADHESIVE	1/4"	900	4"-6"	16" *MINIMUM OF 2 FASTENERS PER UNIT	CONCRETE, BRICK, CONCRETE BLOCK, OR EIFS



**SECTION VIEW:**  
 NTS

**COLORS USED**

Digitally Printed Vinyl	Paint
 PMS 298c (Logo)	 PMS COOL GRAY 10c (Logo)
 PMS 427c (Logo)	 BLACK 100% (Logo)
 PMS 420c (Logo)	 PMS 420c (Face & Returns)

**GENERAL NOTES:**

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  - EXISTING BRANCH CIRCUIT IN COMPLIANCE WITH NEC 600.5, NOT TO EXCEED 20 AMPS
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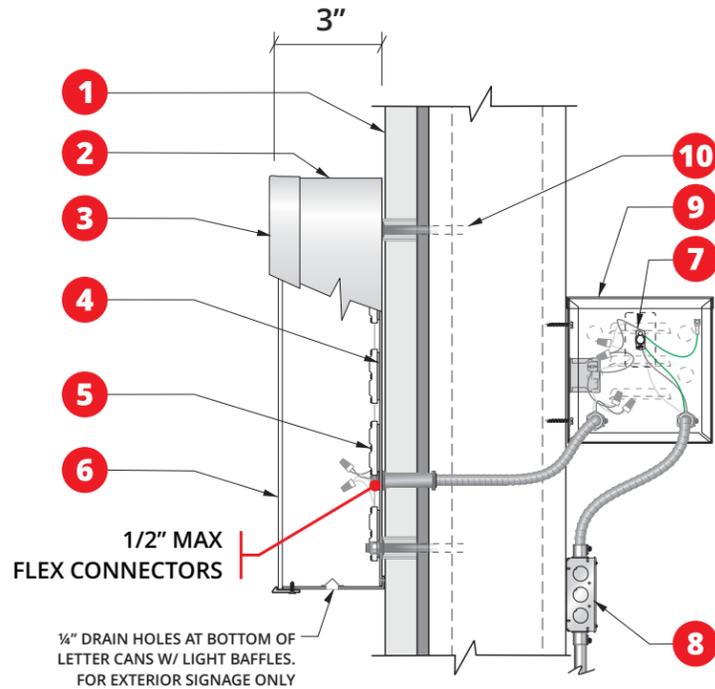
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**FRONT:** 28 SQUARE FEET • **QUANTITY 2**  
**3/8" SCALE**

**SPECIFICATIONS: ILLUMINATED CHANNEL LETTERS**

- 1 EXISTING FACADE
- 2 .040" ALUMINUM LETTER RETURNS PAINTED WHITE
- 3 1" TRIMCAP BONDED TO FACE, #8 PAN HEAD SCREWS TO RETURNS - PAINTED WHITE  
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**SECTION VIEW:**  
 NTS

**COLORS USED**

**WHITE**

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	TYPE	DIA.	SHEAR LBS.	EMBED	SPACING	FOR USE WITH
10	THREADED ROD WITH SILICONE ADHESIVE	1/4"	900	4"-6"	16" *MINIMUM OF 2 FASTENERS PER UNIT	CONCRETE, BRICK, CONCRETE BLOCK, OR EIFS



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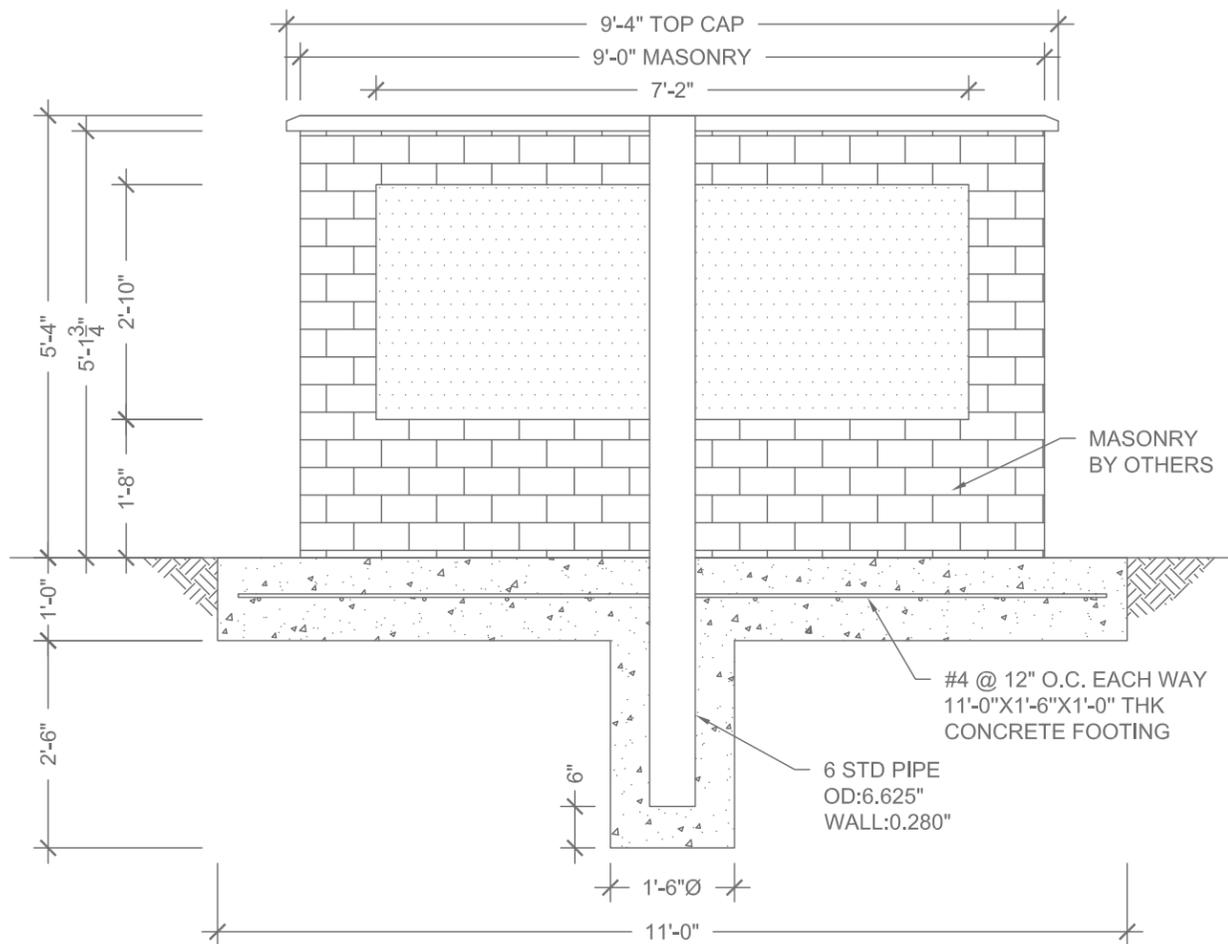
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11545 W. BERNARDO COURT  
SUITE 201, SAN DIEGO, CA 92127  
PROJECTMANAGER@SULLAWAYENG.COM  
PHONE: 1-858-312-5150 FAX: 1-858-777-3534

PROJECT: STRICKLAND BROTHERS - MONUMENT SIGN - 6450 RUFÉ SNOW, NORTH RICHLAND HILLS, TX DATE: 9-14-2023  
PROJECT #: 42353 ENGINEER: JD  
CLIENT: ATLANTIC SIGN COMPANY LAST REVISED:



- GENERAL NOTES**
- DESIGN CODE: IBC 2018
  - DESIGN LOADS: ASCE 7-16
  - WIND VELOCITY: 115 MPH EXPOSURE C
  - CONCRETE 2500 PSI MIN.
  - PIPE STEEL ASTM A53, F<sub>y</sub>= 35 KSI MIN.
  - REINFORCING BAR (REBAR) STEEL ASTM A615 GR. 60 KSI
  - PROVIDE 3" CONCRETE COVER MIN. FOR REBAR
  - PROVIDE PROTECTION AGAINST DISSIMILAR METALS
  - LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
  - VERTICAL SOIL BEARING PER IBC CLASS 4 (2000 PSF)
  - ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION.



9-14-23



11545 W. Bernardo Court, San Diego, CA 92127  
projectmanager@sullawayeng.com  
Phone: 858-312-5150 Fax: 858-777-3534

PROJECT: STRICKLAND BROTHERS DATE: 9/14/23  
PROJ. NO.: 42353 ENGINEER: JD  
CLIENT: ATLANTIC SIGN COMPANY

v5.5 units; pounds, feet unless noted otherwise

**Applied Wind Loads; from ASCE 7-16**

**PIPE/PIER FOOTING**

$F = q_z * G * C_f * A_f$  with  $q_z = 0.00256 K_z K_{zt} K_d V^2$  (29.3.2 & 29.4)  
 $C_f = 1.635$  (Fig. 29.3-1) 1.00 0 max. height= 4.50  
 $K_{zt} = 1.0$  (26.8.2) (=1.0 unless unusual landscape)  $s = 2.83$   
 $K_z =$  from table 28.3-1 Exposure= c  
 $K_d = 0.85$  for signs (table 26.6-1)  
 $V = 115$  mph  
 $G = 0.85$  (26.9) weight= 0.203 kips  
 $s/h = 0.630$   $M_{DL} = 0.00$  k-ft  
 $B/s = 2.53$

Pole Loads	structure component	height at section c.g.	$K_z$	$q_z$	pressure $q_z * G * C_f$	$A_f$	shear	Wind Moment $M_w$	
	1	0.8	0.850	24.5	34.00	0.0	0	0	
	2	3.08333333	0.850	24.5	34.00	20.3	690	2129	
sums:							20.3	690	2.13 ( $M_w$ ) k-ft arm= 3.1
							$P_u = 0.24$ kip	$M = 2.13$ k-ft	$M = \sqrt{M_{DL}^2 + M_w^2}$
							$M_u = \sqrt{1.2 M_{DL}^2 + 1.0 M_w^2} = 2.13$ k-ft		

**Pole Design section; pipe**

$M_u \leq \phi M_n$  with  $M_n = f_y Z$   $f_y = 35$  ksi  $\phi = 0.9$

H	$M_u$ (k-ft)	Z req'd. (in)	Size(in)	t (in)	Z	USE
at grade	2.1	0.81	2.5	0.203	1.4	<b>6 STD PIPE, Mn= 27.8 k-ft</b>

**Footing Design footprint: round**

$\omega = 1.3$  IBC 1605.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2  $S = (1.3x2x)$   
 $P = 0.54$  kip  $S1 = S x d / 3$   $A = 2.34 x P / (S1 x b)$   $S = 400$   
 $S1 = 468$   $d = 0.5xA (1 + (1 + 4.36x h/A)^{.5})$  IBC 1807.3.2.1  
 $A = 1.79$

footing: **1' - 6" dia.** **3' - 6" deep**



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PROJECT: STRICKLAND BROTHERS  
 PROJ. NO.: 42353  
 CLIENT: ATLANTIC SIGN COMPANY

DATE: 9/14/23  
 ENGINEER: JD

---

v5.5 units; pounds, feet unless noted otherwise

### Applied Wind Loads; from ASCE 7-16 MASONRY/SPREAD FOOTING

$F = q_z * G * C_f * A_f$  with  $q_z = 0.00256 K_z K_{zt} K_d V^2$  (29.3.2 & 29.4)

$C_f = 1.416$  (Fig. 29.3-1)  $K_{zt} = 1.0$  (26.8.2) (=1.0 unless unusual landscape)  $K_d = 0.85$  for signs (table 26.6-1)  $V = 115$  mph  $G = 0.85$  (26.9)  $s/h = 1.000$   $B/s = 1.69$

$K_z =$  from table 28.3-1 Exposure = c  $M_{DL} = 0.00$  k-ft  $weight = 0.278$  kips

Pole Loads	structure component	height at section c.g.	$K_z$	$q_z$	pressure $q_z * G * C_f$	$A_f$	shear	Wind Moment $M_w$
	1	0.8	0.850	24.5	29.43	15.0	442	368
	2	3.08333333	0.850	24.5	29.43	5.2	153	471
	3	4.82291667	0.850	24.5	29.43	5.8	171	825
	4	5.23958333	0.850	24.5	29.43	1.8	52	270
					sums:	27.8	817	1.93 (M <sub>w</sub> ) k-ft arm = 2.4
					for s/h=1, add 10% (asce fig. 29.4-1):	x 1.10		2.13
					$P_u = 0.33$ kip		$M = 2.13$ k-ft	$M = \sqrt{M_{DL}^2 + M_w^2}$
					$M_u = \sqrt{1.2M_{DL}^2 + 1.0M_w^2} = 2.13$ k-ft			



P.O. Box 28789, SD, CA 92198  
 mikesullaway@gmail.com  
 Phone: 858-312-5150 Fax: 858-777-3534

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DATE: 9/14/23  
 ENGINEER: JD

---

Version 3.0 units; pounds, feet unless noted otherwise

### Longitude Direction

applied shear at grade  $v = 0.49$  kip unfactored load 0.817 k LRFD  
 applied moment at grade  $m = 1.278$  kip-ft unfactored load 2.13 k-ft LRFD  
 depth of soil above footing  $h_s = 0$  ft  
 allowable soil bearing  $p = 2.0$  ksf  
 weight of CMU cap  $w_1 = 0.346$  k (9'-4"x0'-2 1/4"x1'-7")x0.125kcf  
 weight of masonry base  $w_2 = 4.334$  k ((2'-3 3/4"x9'-0")+(2'-10"x1'-10"))x1'-4"x0.125kcf  
 Total weight  $w = 4.7$  k (w<sub>1</sub>+w<sub>2</sub>)

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### Spread Footing Design

moment  $m = 1.8$  k-ft at base of footing  
 Footing size (ft)  $b = 11.00$   $L = 1.5$   $h = 1.00$   $S = 4.1$   
 Footing Weight = 2.5 k signage weight = 4.7 k soil 0.00 total = 7.16  
 Overturning;  $M_c = 5$   $M_c > 1.5M$  3.0352 ok  
 soil pressure; max = 0.862 ksf ok  
 forces on concrete pad;  $V = 5.3$  k  $V_r = 9$  k (=1.6V)  
 $M = 2$  k-ft  $M_r = 3$  k-ft

**Check Slab;**  
 $\phi M_n = \phi A_s f_y (d - a/2) = 48$  k-in = 4.04 k-ft  $M_r < \phi M_n$  ok  
 $a = A_s f_y / 0.85 f_c b = 0.032$  in  
 Flexure  $A_s = 0.15$   $f_y = 60$  ksi  $f_c = 2.5$  ksi 150 lbs/ft3  
 $d = 6.0$  in

**Check minimum**  $A_{smin} = 2 \sqrt{f_c} b d / f_y = 1.98$  200bd/fy = 2.64 or 1.333A<sub>s</sub> = 0.20 in<sup>2</sup>  
 short direction  $\gamma_s = 2 / (\beta + 1) = 0.8$  with  $\beta = 1.5$  short direction;  $\gamma A_s = 0.16$  in<sup>2</sup>  
**USE #4 @ 12" Each Way**

Shear;  $\phi V_n = \phi 2 \sqrt{f_c} b d$   $\phi V_c = 59.4$  = 0.75  $V_r < \phi V_n$  ok